

Roundabout Design 101: Roundabout Capacity Issues

Part 2
March 7, 2012



Presentation Outline – Part 2

- Geometry and Capacity
- Choosing a Capacity Analysis Method
 - Modeling differences
 - Capacity
 - Delay
 - Limitations
- Variation / Uncertainty in Prediction
- Examples
- Staging Construction to Match Volume Increase



Geometry Strongly Affects Capacity

Comparison of Traffic Signals and Roundabouts In Terms of How We Add Capacity

Traffic Signals

- A combination of geometry and timing plans
- Capacity added in lane increments

Roundabouts

- Rely primarily on geometry
- Subtle changes produce the largest benefits when conditions appear unmanageable



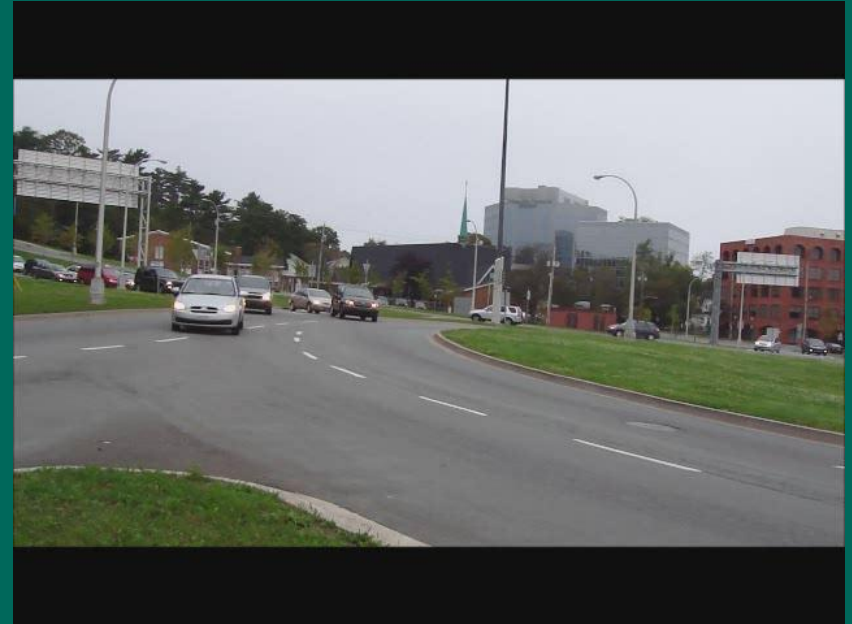
Geometry Affects Capacity

Poor Geometry



Before

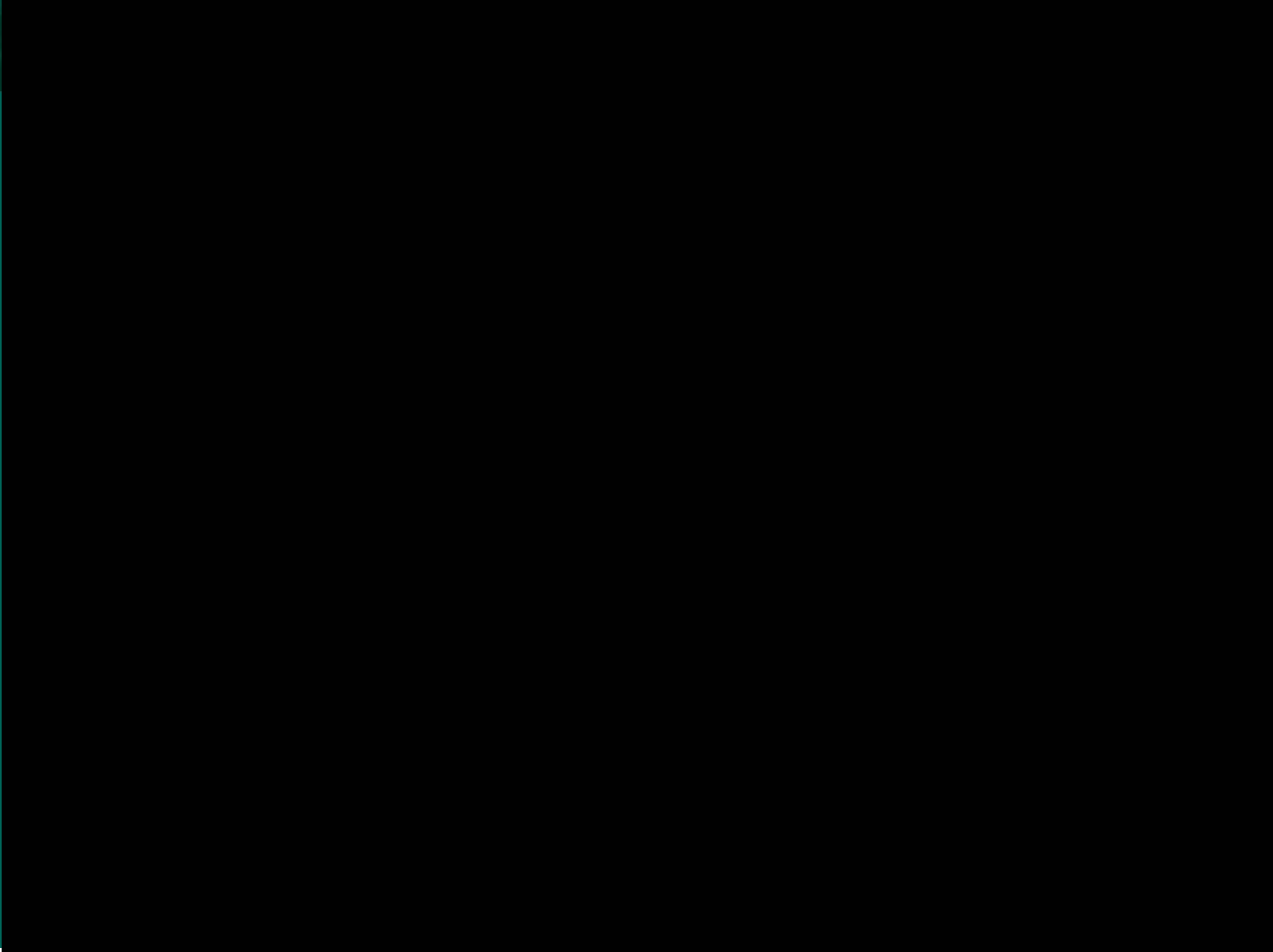
Improved Geometry



After



Roundabout Performance with Tidal Flows



Types of Capacity Models for Modern Roundabouts

Linear Regression (strong empirical basis in U.K.)

- Arcady, RODEL – TRL Report 942 equations

Gap Acceptance

- Sidra
- HCM 2010

Theoretical Models (less empirical basis):

- Vissim, Paramics, Synchro, Aimsun



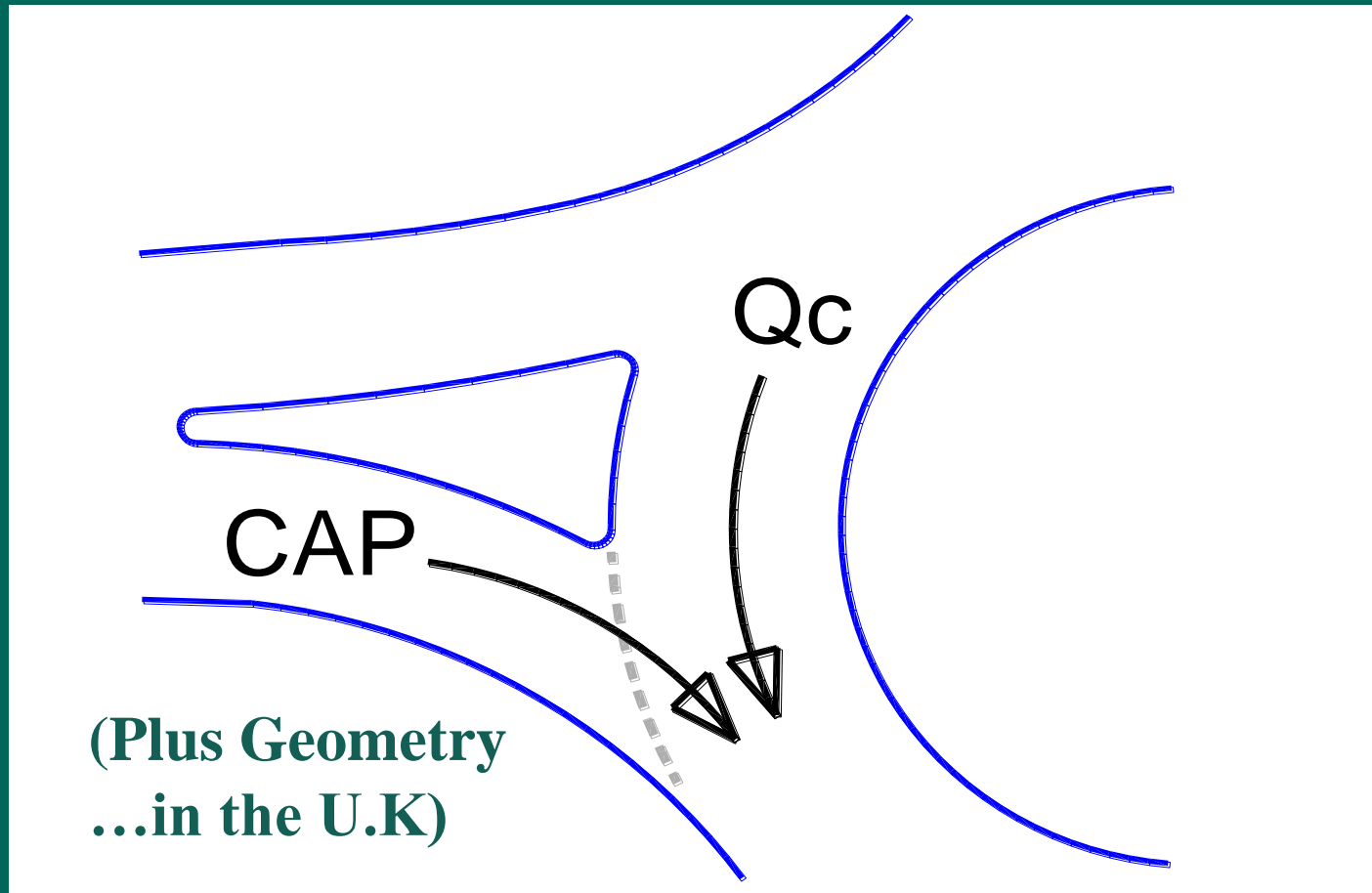
New FHWA Guide

Chapter 4 – Selection of Analysis Tools

| Application | Typical Outcome Desired | Input Data Available | Potential Analysis Tool |
|---|--|--|--|
| Planning-level sizing | Number of lanes | Traffic volumes | Section 3.5 of this guide, HCM, deterministic software |
| Preliminary design of roundabouts with up to two lanes | Detailed lane configuration | Traffic volumes, geometry | HCM, deterministic software |
| Preliminary design of roundabouts with three lanes and/or with short lanes/flared designs | Detailed lane configuration | Traffic volumes, geometry | Deterministic software |
| Analysis of pedestrian treatments | Vehicular delay, vehicular queuing, pedestrian delay | Vehicular traffic and pedestrian volumes, crosswalk design | HCM, deterministic software, simulation |
| System analysis | Travel time, delays and queues between intersections | Traffic volumes, geometry | HCM, simulation |
| Public involvement | Animation of no-build conditions and proposed alternatives | Traffic volumes, geometry | Simulation |



Capacity of An Approach



HCM Multilane and Critical Lane Capacity

1. Multilane entry conflicted by one circulating lane

- Apply single-lane capacity model to critical approach lane

$$c = 1130 \cdot \exp(-0.0010 \cdot v_c) \quad (4-4)$$

where

$c = q_{e,max}$ = entry capacity (veh/h)

$v_c = q_c$ = conflicting circulating traffic (pcu/h).

1. Multilane entry conflicted by two circulating lanes:

- Apply NCHRP 572 multilane capacity model to critical approach lane

$$c_{crit} = 1130 \cdot \exp(-0.0007 \cdot v_c) \quad (4-7)$$

where

$c_{crit} = q_{e,max,crit}$ = capacity of the critical lane (pcu/h)

$v_c = q_c$ = conflicting flow (pcu/h)



Advantages of HCM Model

- Familiar to analysts with stop-controlled intersections
- Straight-forward and outlined in the 2010 HCM
 - ability to analyze two lane roundabouts
- Able to calibrate HCM equations by using only one other site's gap parameter data
- Based on U.S. data



HCM Limitations

- Queue and delay predictions overlapping time slices require iterative analysis (limit ~ 0.85 v/c)
- Closely spaced intersections
- Three lane entries
- No geometric variation for complex design
- Need to look back at built roundabouts to see what HCM would have revealed for capacity requirements – many examples to choose from



Which Method to Use?

- HCM method is becoming customary, but there are limitations...
- How to meet agency's desire for HCM model results but still provide confidence in the analysis?

Multiple analyses are becoming the norm to demonstrate confidence in the prediction.

* Especially for complex designs

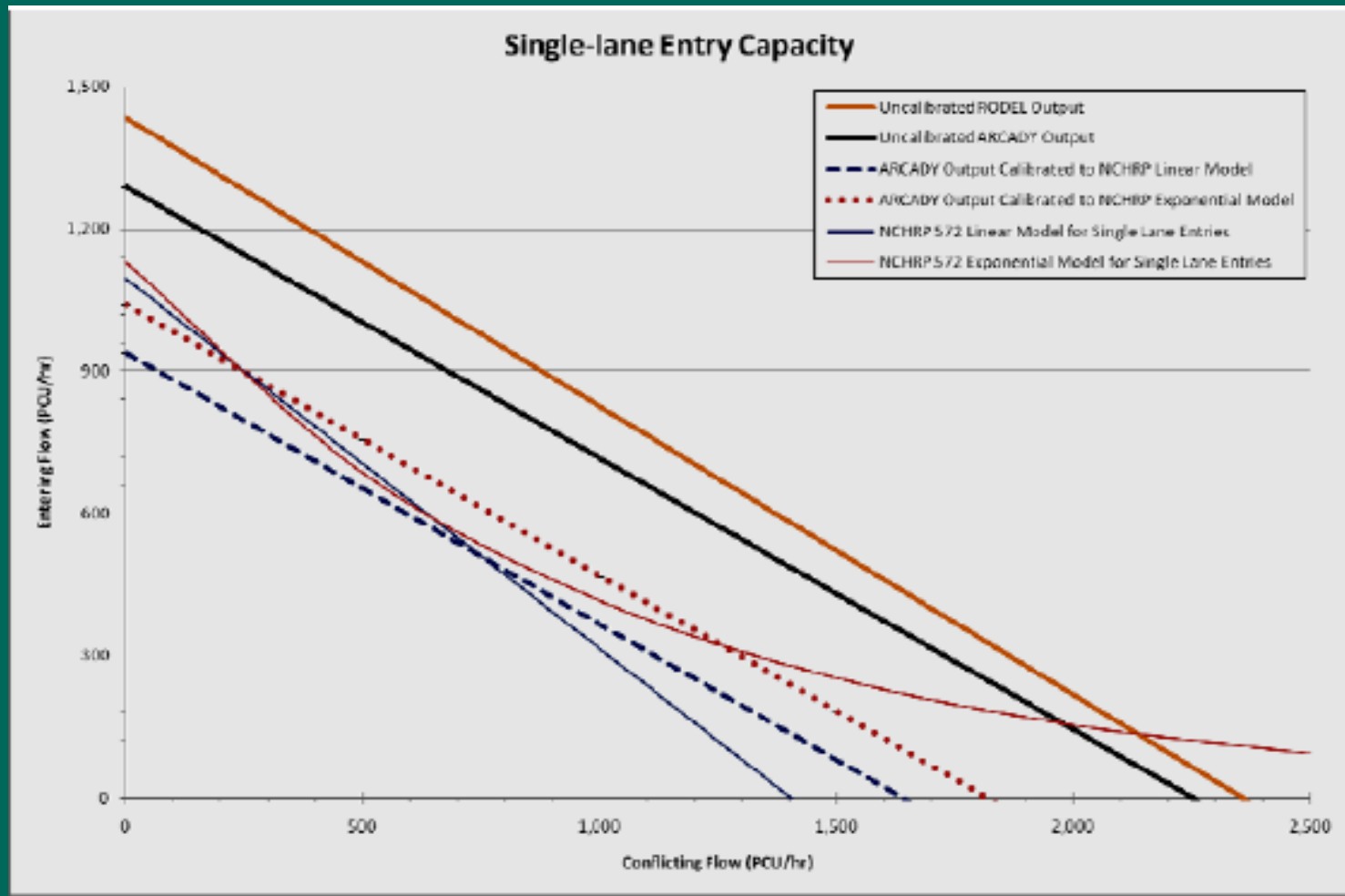


Current Trend Among Analysts

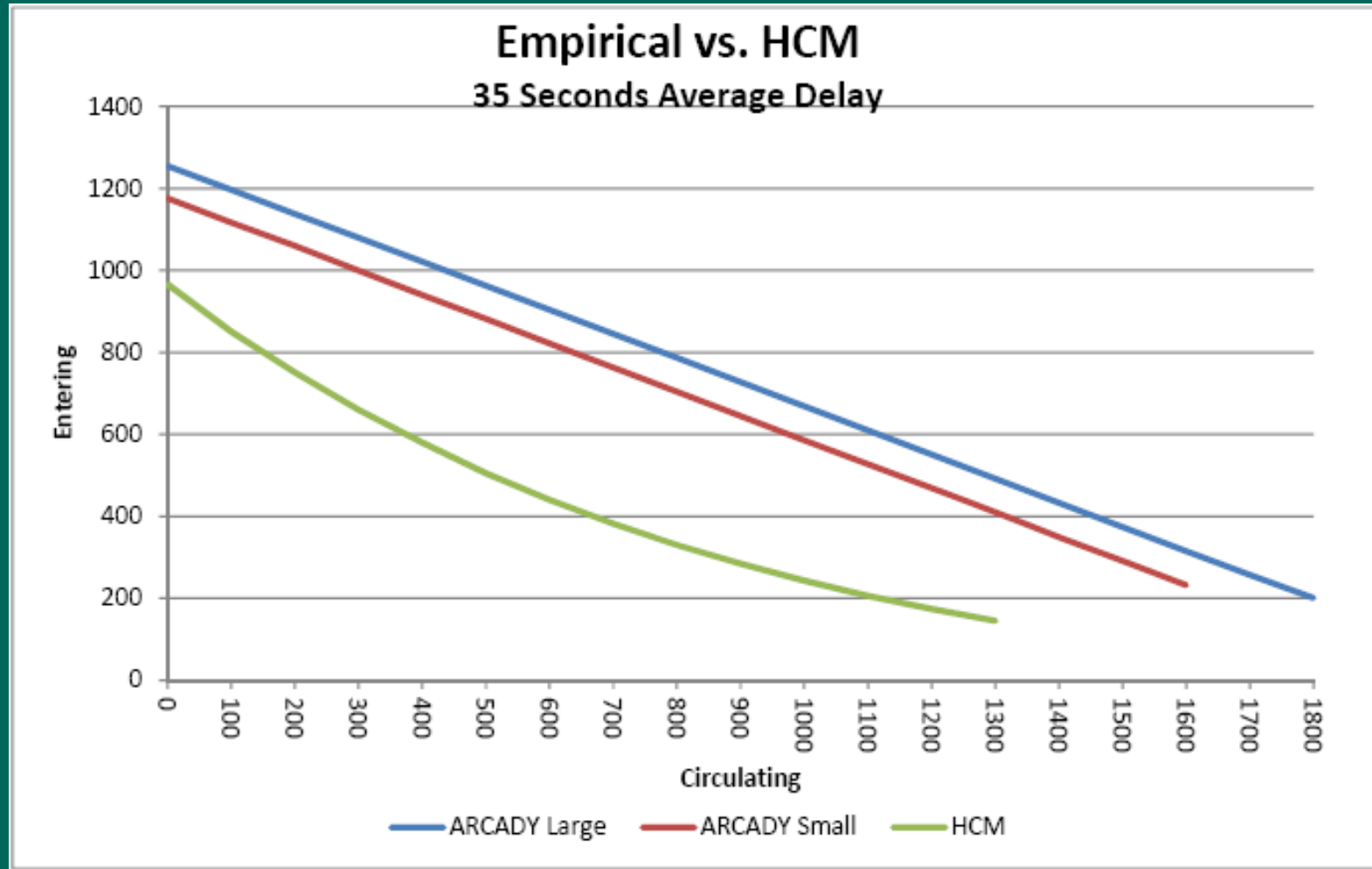
- Use multiple models to report a range of results
- Conduct sensitivity testing by:
 - Varying traffic flow
 - Calibration using out-of-state values
 - Consideration of staged design...1st 10 years...
- Other states are responding similarly, e.g. GA DOT mandates HCM but allows for Sidra and Arcady results to supplement



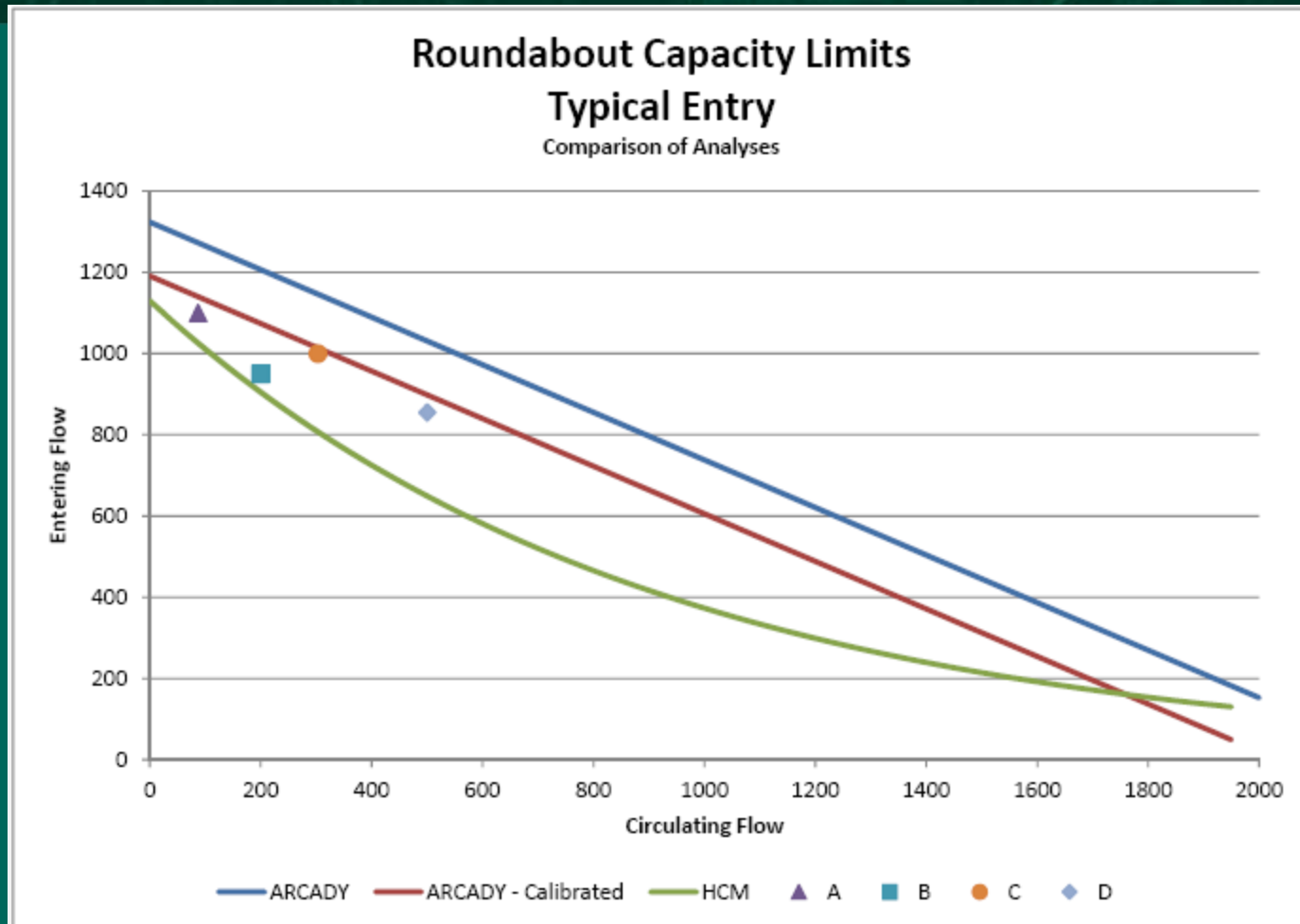
Variation in Capacity Prediction Between Models

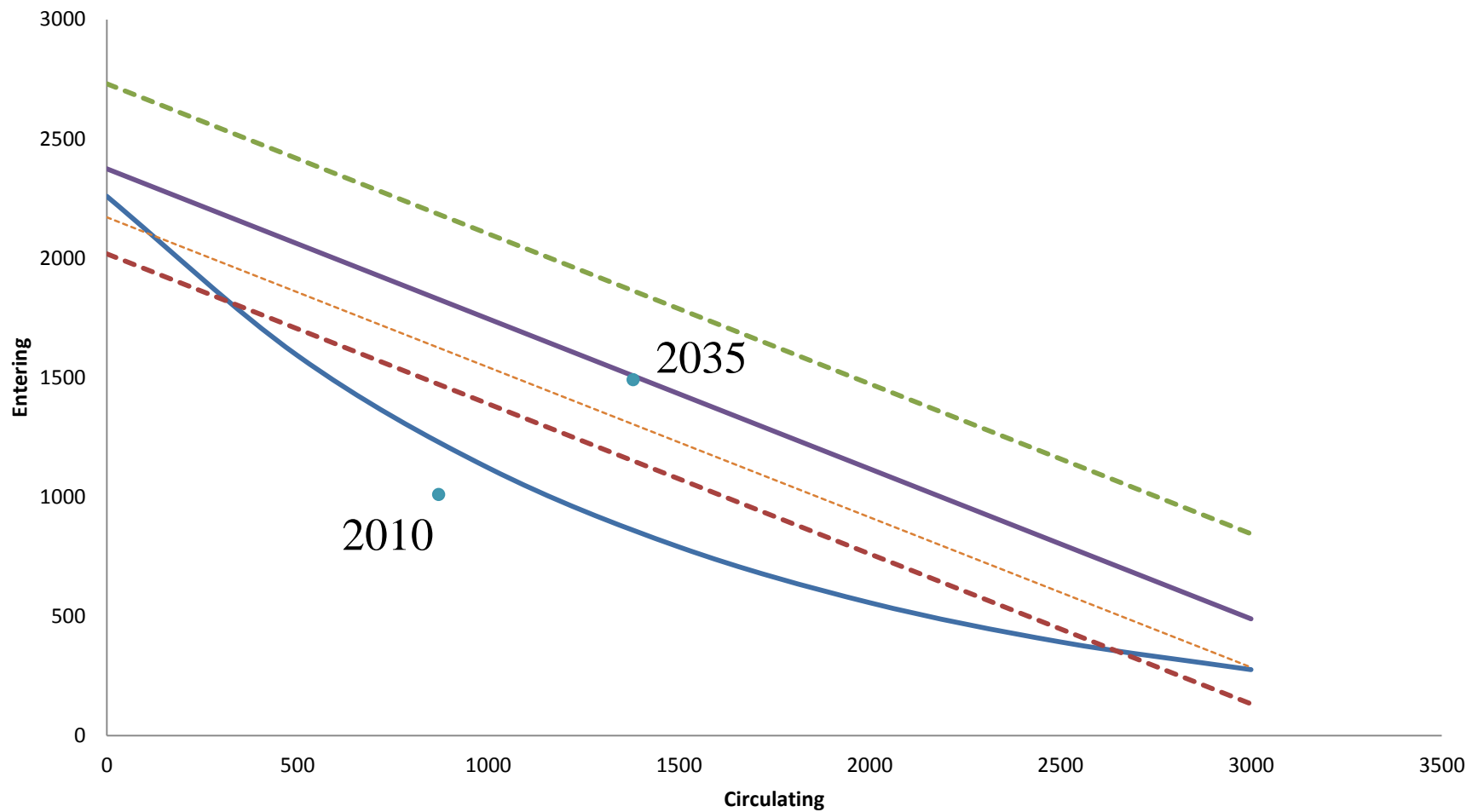


Variation in Delay Prediction Between Models



Example Comparison of Analyses





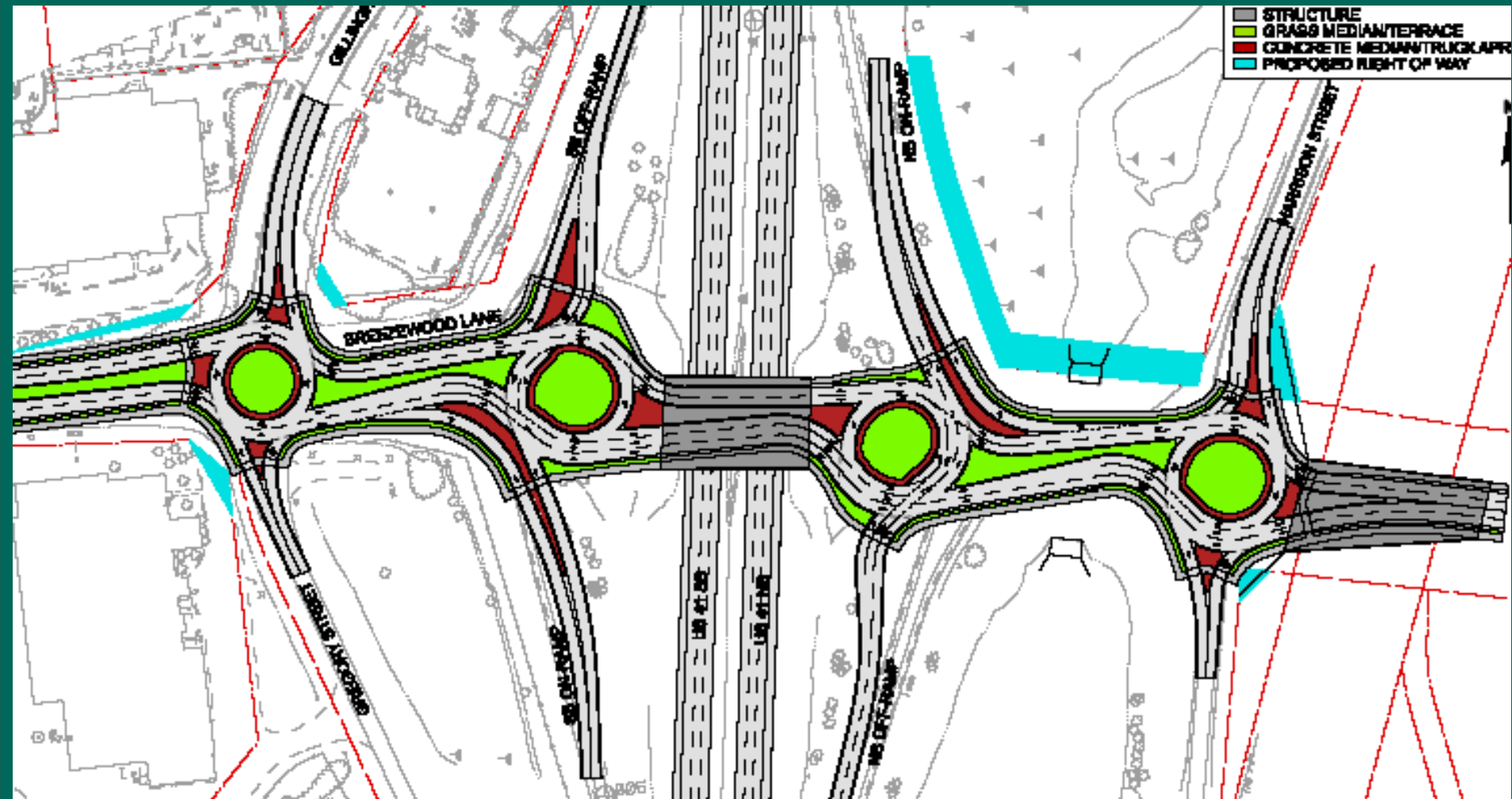
— HCM — U.K. - - U.K. - 15% - - U.K. + 15% • PM - - U.K. 85% CL

How Certain are the Inputs?

- Traffic projections
- Land development / re-development
- Operation prediction models
- Driver behavior
- Vehicle characteristics
- Existing traffic flows – often the only indisputable input



The Complexity Increases as Capacity is Underestimated...



Field Measurement of Capacity

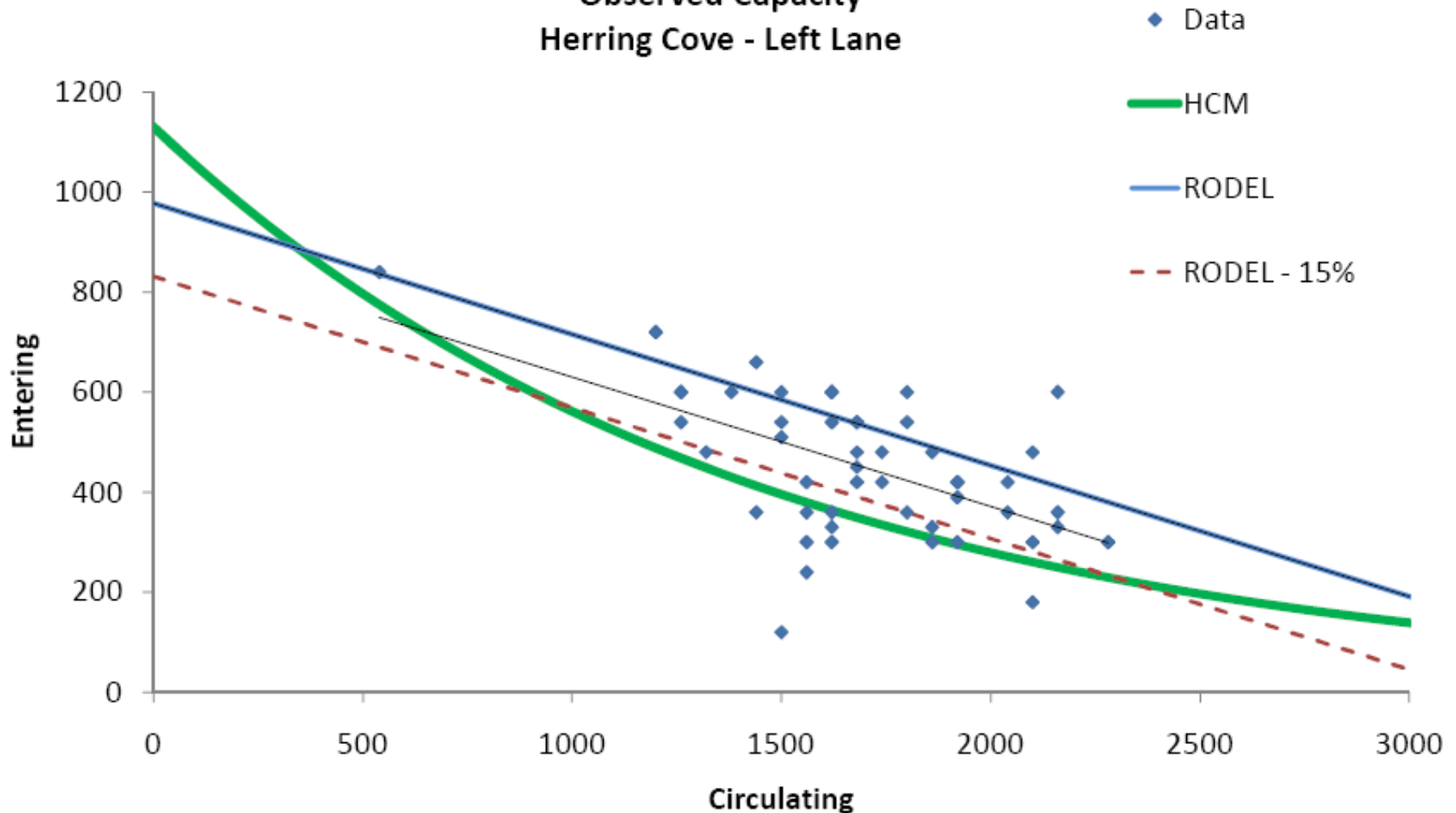
- Entry must have sustained queuing
 - Minimum 4-6 vehicles
- Count entering and circulating volumes
- Capture a wide range of flow variations
- Look for features that influence flow





Observed Capacity Example

Armdale Roundabout
Observed Capacity
Herring Cove - Left Lane



Why Build Single-Lane Initially?

- Operational simplicity, especially if first roundabout in area
- Cost is lower initially (although higher in total)
- Single-lane roundabouts exhibit better safety than multi-lane roundabouts
- Without volumes to justify multi-lane, more potential for drivers to ignore lane markings
 - resulting in higher speeds

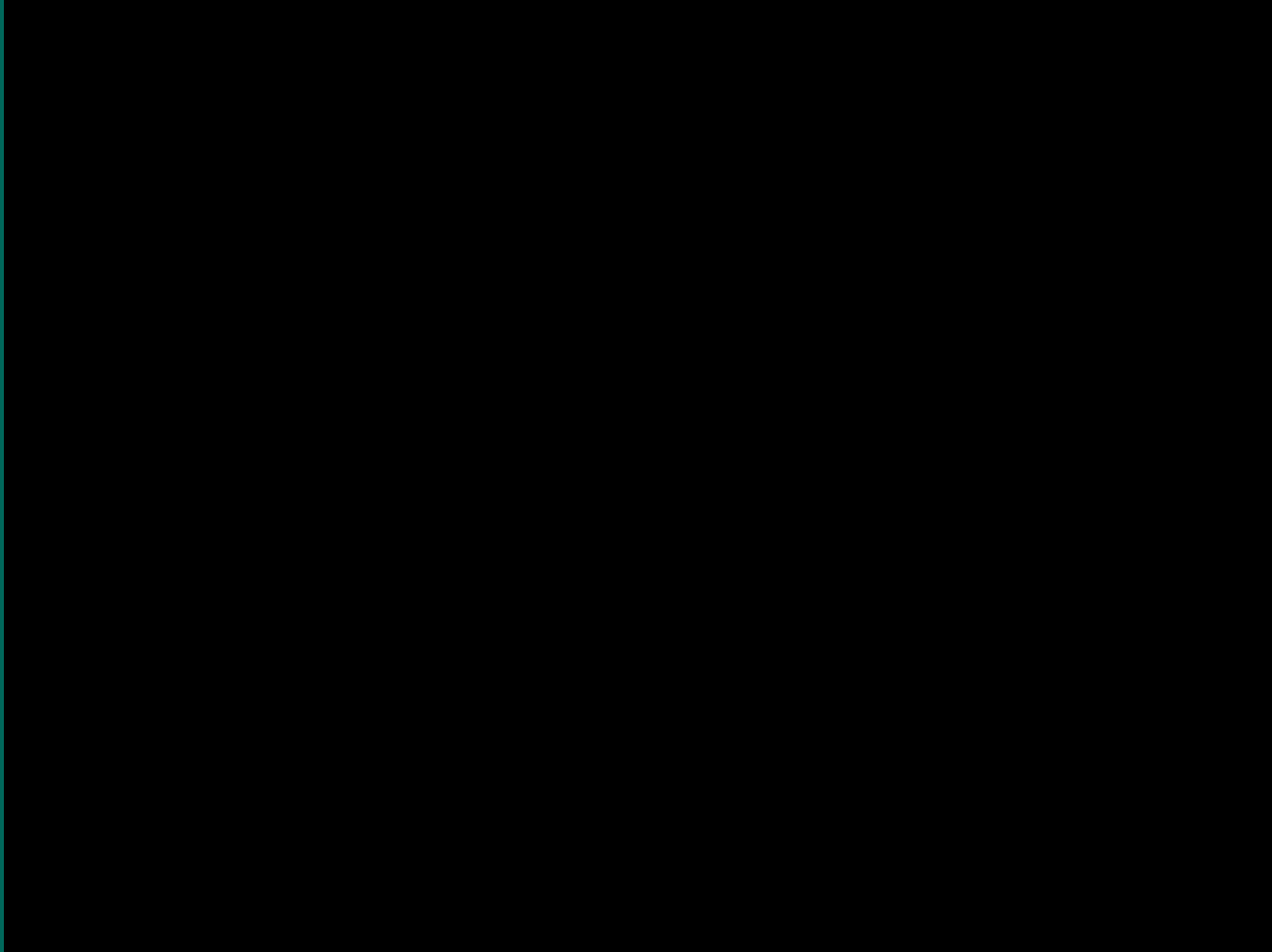


Timing of the Ultimate Layout?

- Can eliminate the need to reconstruct intersection when road corridor is widened



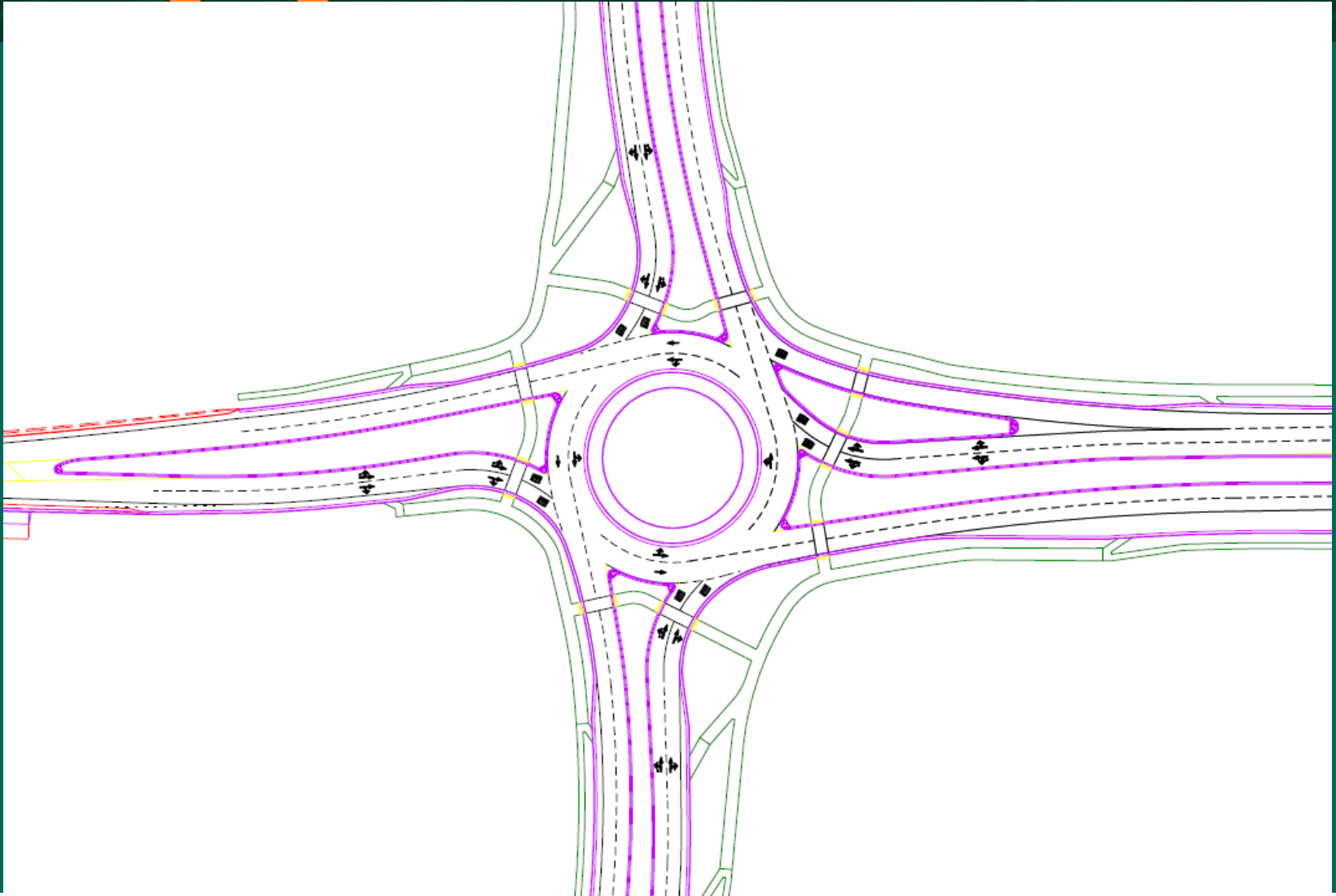
2 Lanes to 1 Exit Merging



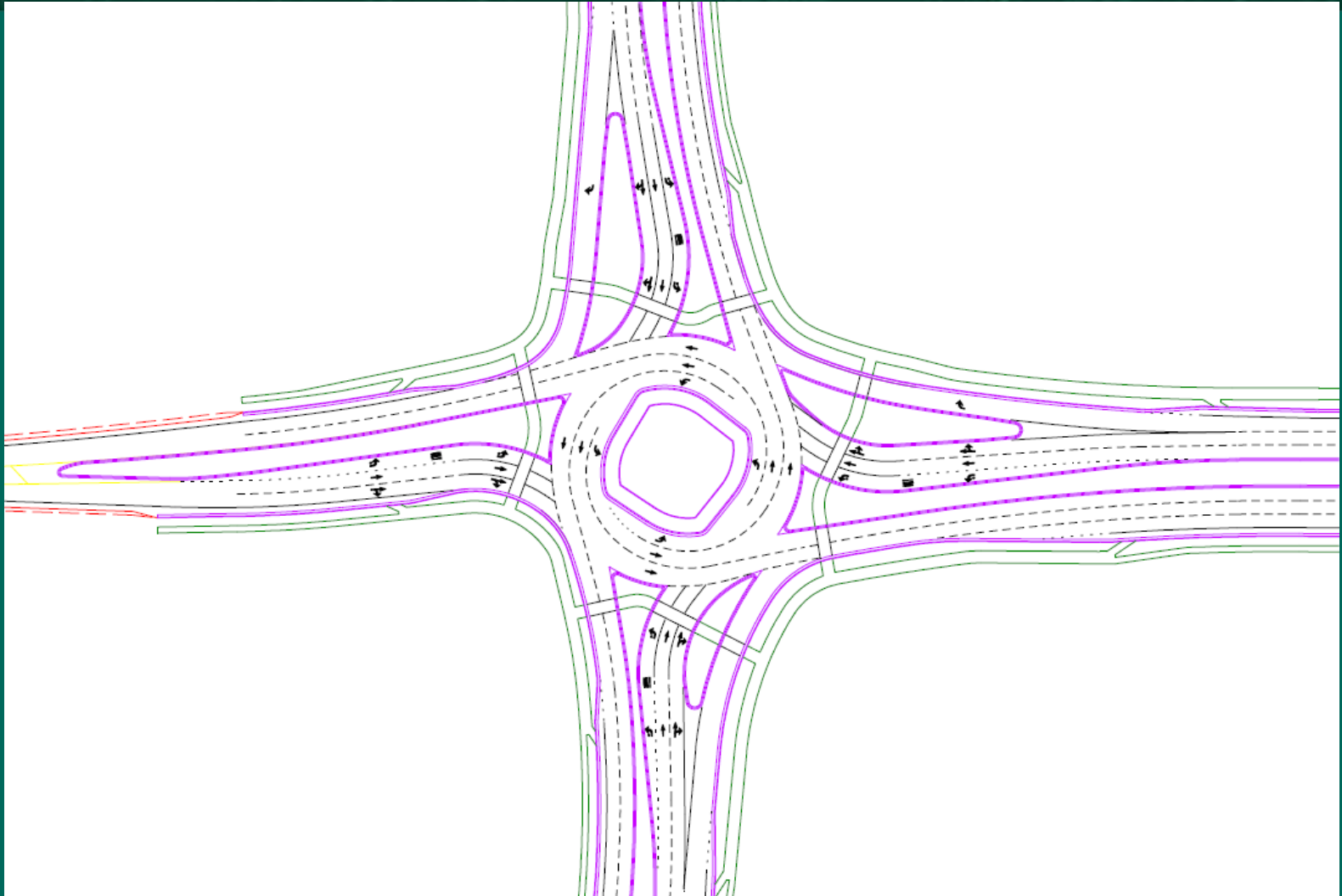
20 Year Roadway, 75 Year Bridge



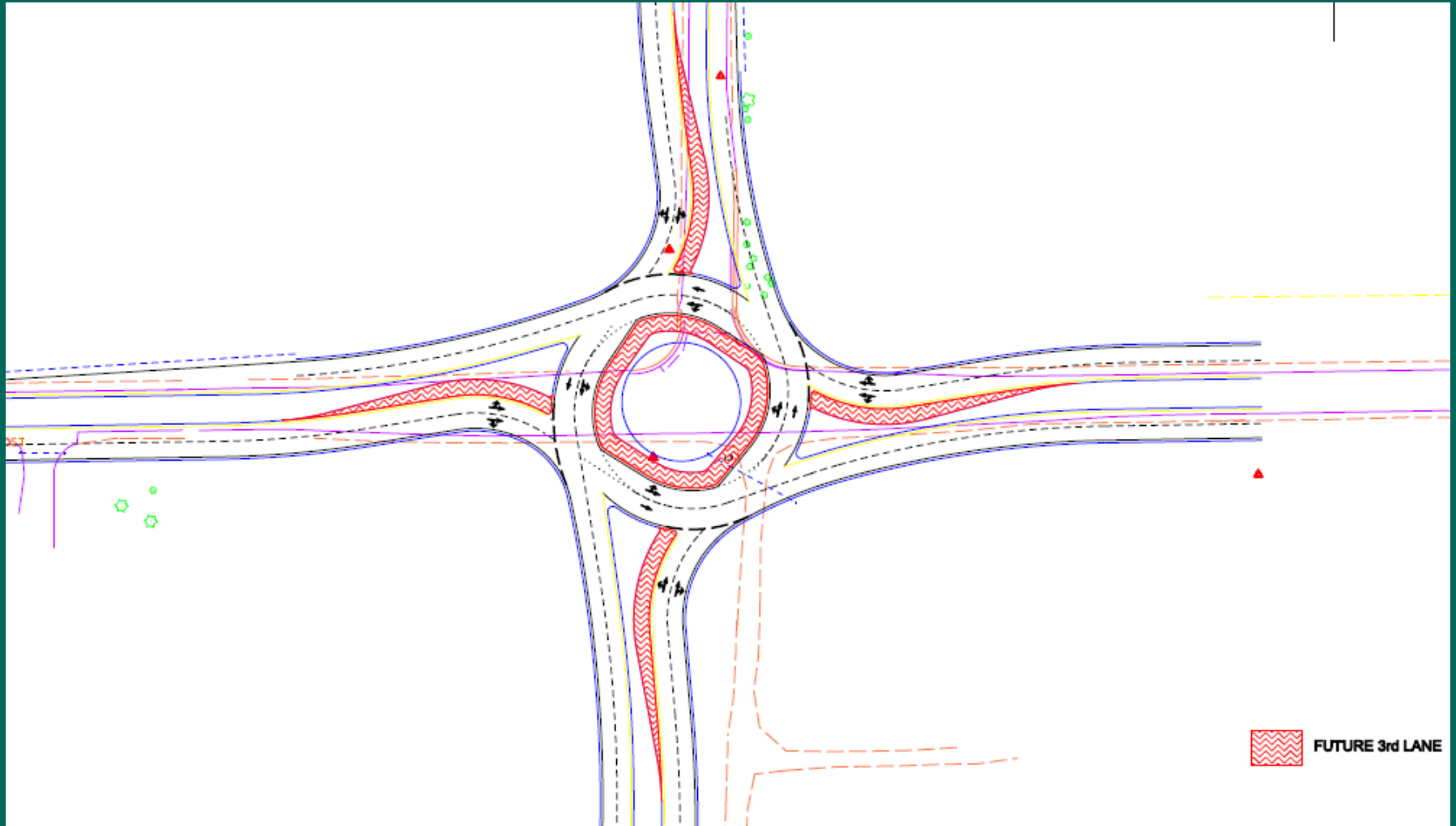
Staging Inward – 2 Lane, Build



Staging Inward – 3 Lane, Ultimate



Staging Inward

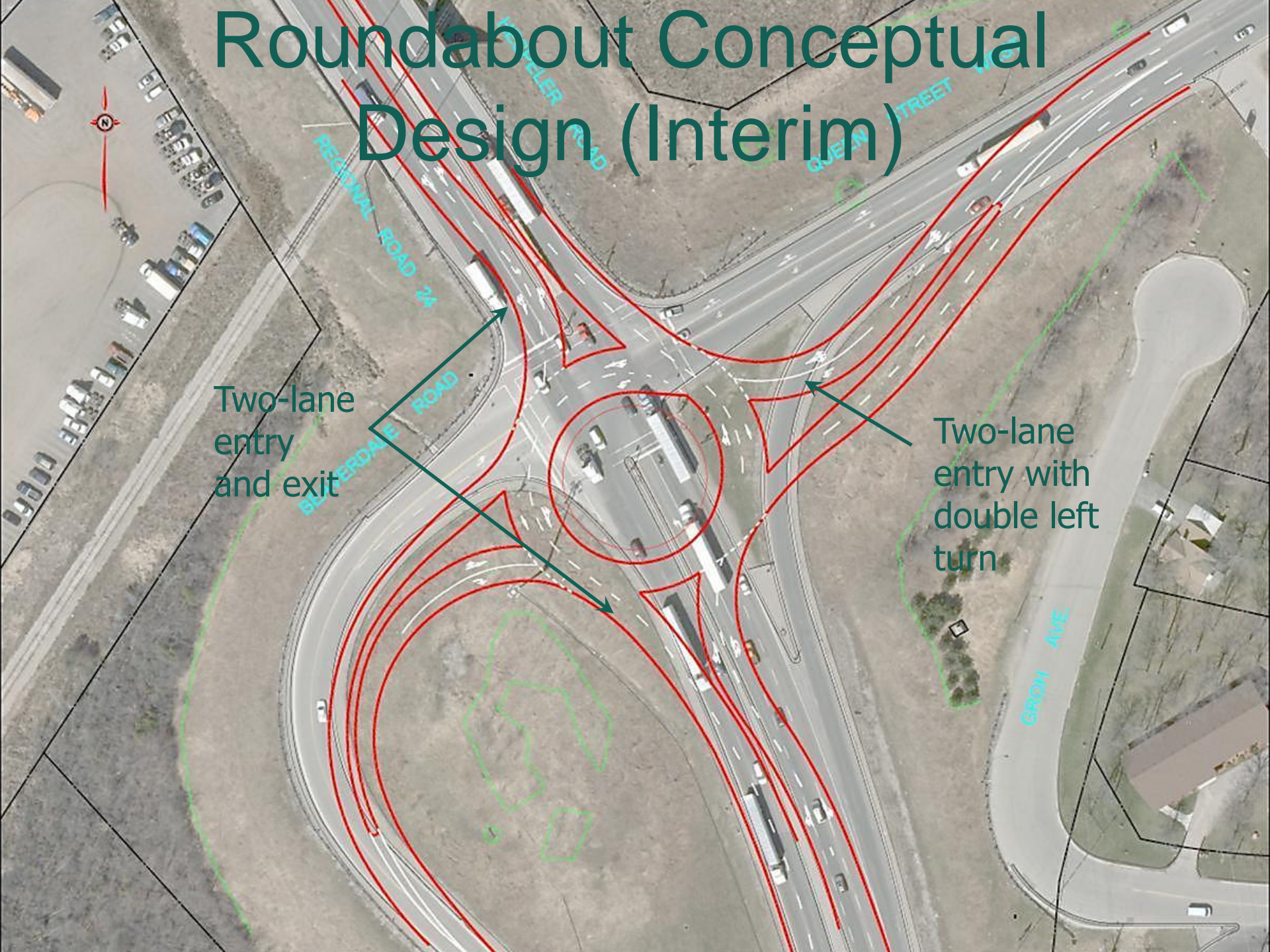


Staging Outward

- Build final central island and splitter islands and widen outward
- Advantages:
 - Smaller roundabout initially
 - Easier to design, and some markings can often be salvaged
 - Simpler with non-symmetric widenings (i.e. northbound only, instead of northbound and southbound)
 - Lower initial cost



Roundabout Conceptual Design (Interim)



Two-lane
entry
and exit

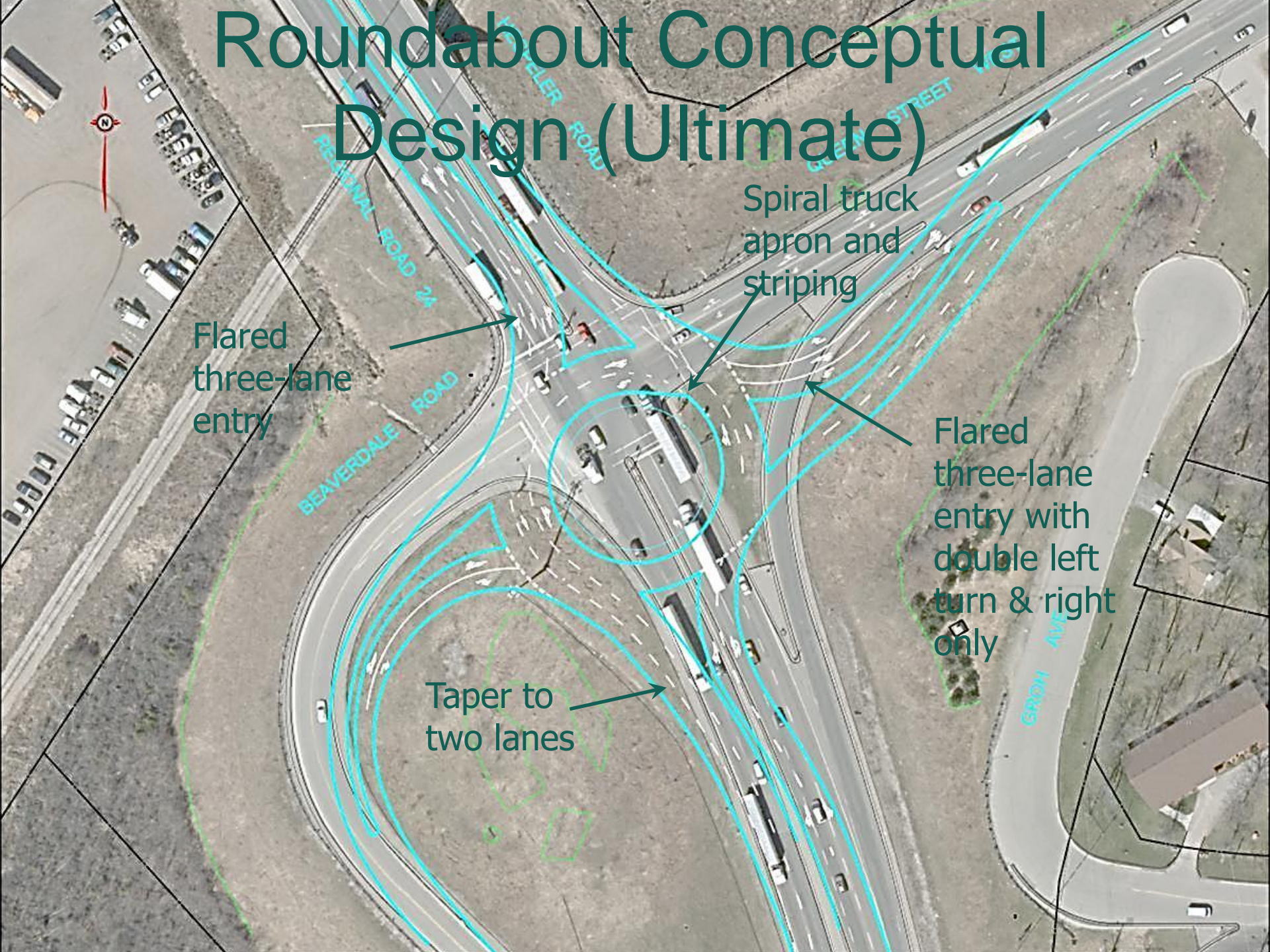
Two-lane
entry with
double left
turn

Staging Outward

- Disadvantages:
 - Adjoining properties disturbed twice instead of once
 - Relocation of drainage features, sidewalks and bike lanes
 - Higher overall cost



Roundabout Conceptual Design (Ultimate)



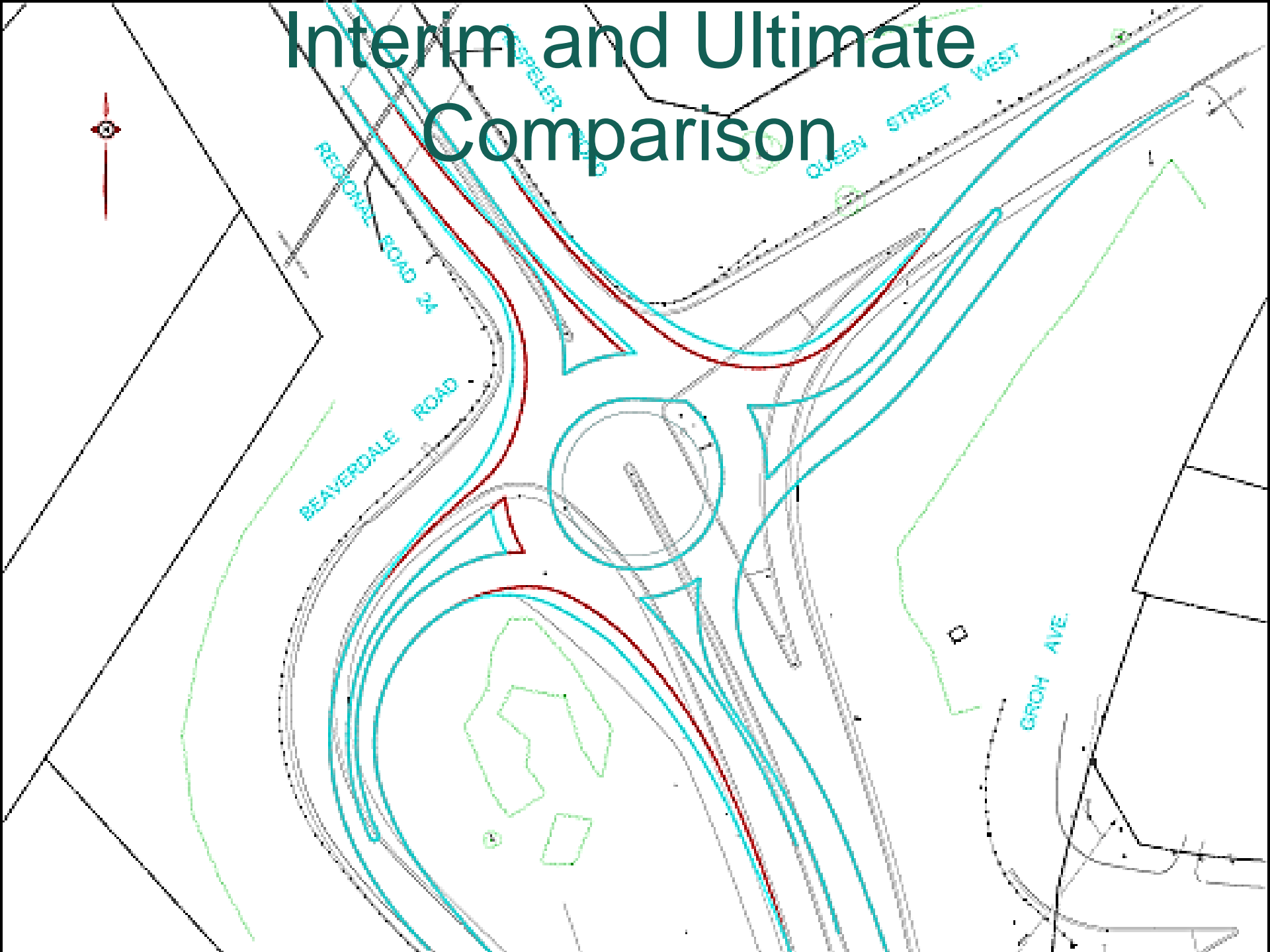
Flared
three-lane
entry

Spiral truck
apron and
striping

Flared
three-lane
entry with
double left
turn & right
only

Taper to
two lanes

Interim and Ultimate Comparison



Theoretical Models

What situations are appropriate for simulation?

- Typically used as an additional capacity analysis tool (not primary)
- Interaction between roundabouts and signalized intersections
- Merging and weaving analysis
- Roundabout system interaction
- Very effective tool for public presentations



Micro-Simulation Example



“Seeing is Believing?”

- Be cautious of “Seeing is Believing” attitude
- Have seen VISSIM simulation with pedestrians walking through cars.
- Have seen vehicles drive through trucks
- Have seen two semi-trucks simultaneously enter a roundabout with out encroaching on adjacent lanes (not realistic)
- Use evidence based approach; Actual field statistics for engineering work. Statistics describe empirical data (reality)



Including Simulation with Analysis

Advantages:

- Typically used as an additional capacity analysis tool (coupled with a design tool like Arcady)
- Interaction between roundabouts and signalized intersections
- Merging and weaving analysis
- Roundabout system interaction
- Very Effective tool for public presentations

Disadvantages:

- Calibration is very difficult
- Uniformity among users is elusive (too many bells...)





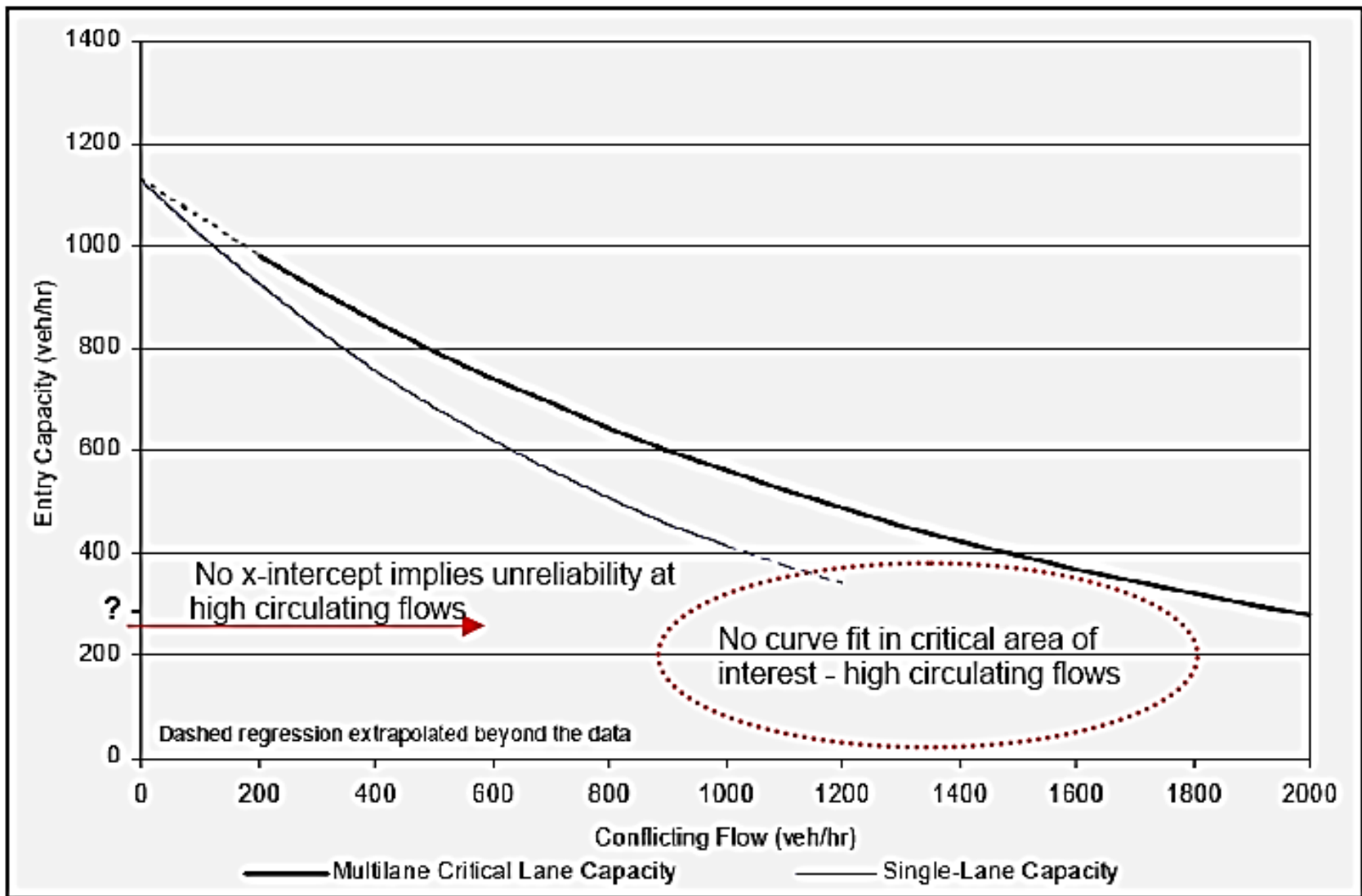
**Ourston
Roundabout
Engineering**

Alternative Capacity Analysis Methodology

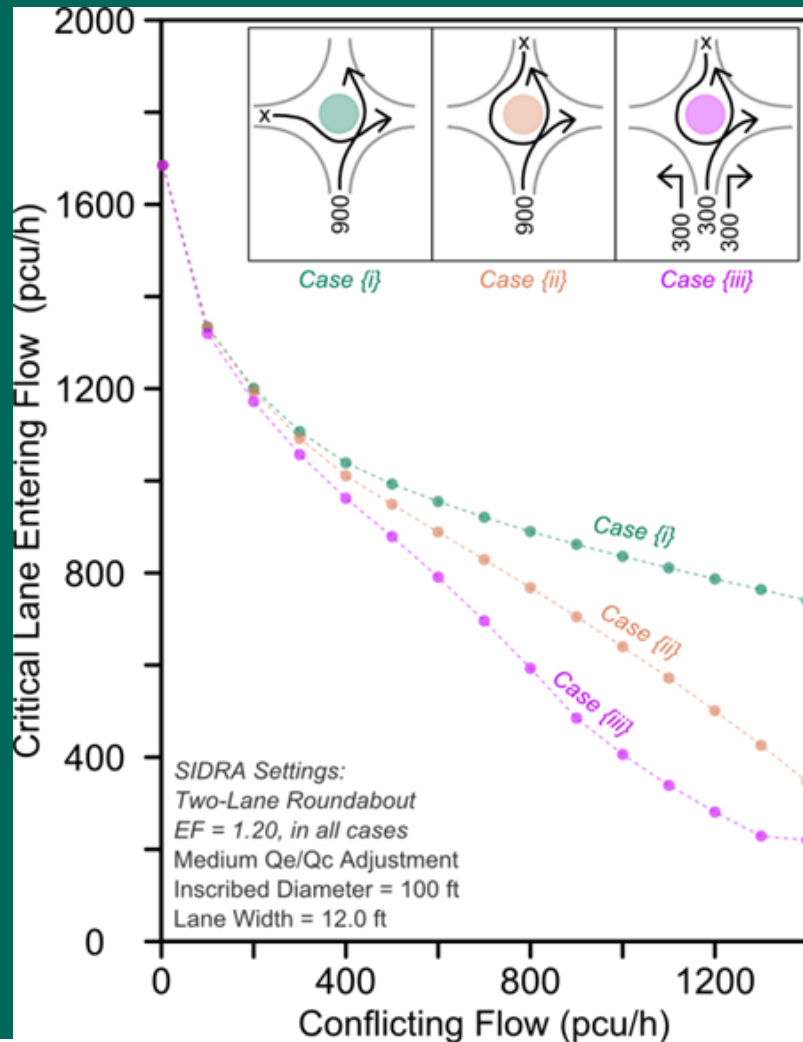
- Empirical
 - ARCADY
 - RODEL
- Gap-Acceptance
 - Sidra
 - HCM
- Micro-Simulation
 - Vissim, Paramics, Aimsun



U.S. Capacity Model



Capacity Models in SIDRA



- Origin-Destination patterns affect capacity results
 - *These three cases show **extreme** examples to illustrate O-D effects
1. Capacity of South Leg was analyzed, varying the number of turning vehicles while maintaining 900 vehicles entering
 2. Circulating volumes were increased for either the upstream thrus or opposing lefts, as depicted on the graph, to create conflicting flows in front of the South Leg
 3. Capacity results for the critical entering lane of the South Leg were recorded as entering flow and plotted against the conflicting flow from Step 2



Delay Models in SIDRA

- Two standard delay models for roundabouts
 - SIDRA Standard Delay
 - Delay results in summary tables include geometric delay
 - Can obtain “Stop Line” delay from detailed output, which is claimed to be comparable to control delay reported by HCM 2010 Delay Formula
 - HCM 2010 Delay
 - Delay is calculated with HCM 2010 Delay Formula
 - Delay results in summary tables are Control Delay, geometric delay is not included



SIDRA Calibration Options

1. Environment Factor

- (1.0, 1.1 and 1.2...2.0; higher values imply more restriction)

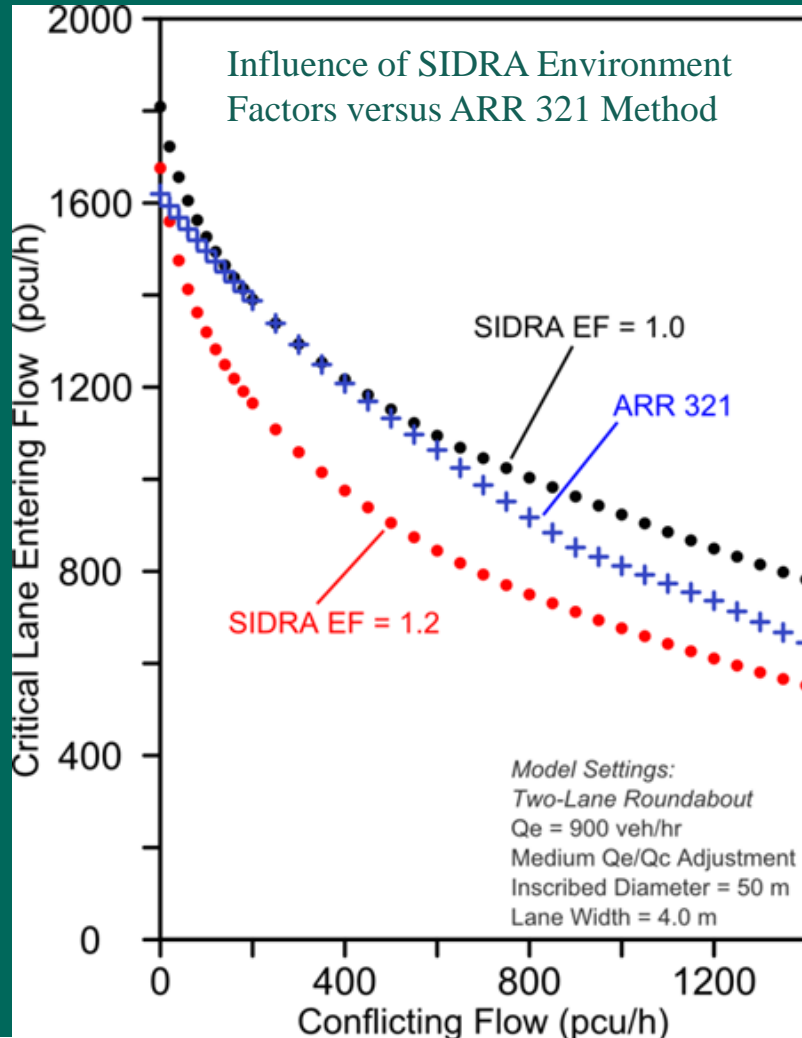
2. Entry/Circulating Flow Adjustment

- High, medium and low settings

3. Critical Gap and Follow-up Headway



SIDRA Calibration Options



1. Environmental Factor (EF)

- EF adjusts critical gap and follow-up headway parameters to account for general roundabout environmental terms
- Can only be changed when using SIDRA Standard Capacity Model
- User guide suggests using EF 1.2 to calibrate capacity to observed US data



SIDRA Calibration Options

2. Entry/Circulating Flow Adjustment

- Adjustment of critical gap and follow-up headway parameters to avoid underestimation of capacities at low circulating flows
- None, Low, Medium, High
 - None results in lowest capacity results
 - High results in highest capacity results
- Default for SIDRA Standard Capacity Model is Medium
- Default for US HCM 2010 Capacity Model is None

3. Critical Gap and Follow-up Headway

- If local values from field data are known, parameters can be changed in the Gap Acceptance window



Corridor Widenings

- Flared entries at roundabouts extend 30-300ft. upstream
- Taper and parallel deceleration lanes at signalized intersections require 450-600ft.
- Because of these extensive lengths, widening the entire corridor under traffic signal control often costs little more
- The ROW impacts may be the same because of utilities



Variation in Capacity Prediction Between Models

- Show comparison graph from TOPS report

